

**NORTH DAKOTA
DEPARTMENT OF TRANSPORTATION**

**MATERIALS AND RESEARCH
DIVISION**

Experimental Study ND 00-01

**Six - Cell Seal
Elastic Sealer**

Second Evaluation Report

Project IM-8-029(026)053
Project IM-5-094(018)059

February 2003

Prepared by

**NORTH DAKOTA DEPARTMENT OF TRANSPORTATION
BISMARCK, NORTH DAKOTA
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**DIRECTOR
David A. Sprynczynatyk, P.E.**

**MATERIALS AND RESEARCH DIVISION
Ron Horner**

EXPERIMENTAL PROJECT REPORT

EXPERIMENTAL PROJECT	EXPERIMENTAL PROJECT NO.					CONSTRUCTION PROJ NO	LOCATION
	1	STATE ND	YEAR 2000	NUMBER -	SURF 01	IM-5-095(018)059	Cass and Stark 28 Counties
						8	
	EVALUATION FUNDING					NEEP NO.	PROPRIETARY FEATURE?
	48	1	X	HP&R	3	DEMONSTRATION	X Yes
		2		CONSTRUCTION	4	IMPLEMENTATION	No
					49		51
SHORT TITLE	TITLE 52 Six-Cell Delastic ®Sealer						
THIS FORM	DATE	MO.	YR.	REPORTING			
	140	02	--	2003	1 INITIAL	2 X ANNUAL	3 FINAL
KEY WORDS	KEY WORD 1			KEY WORD 2			
	145 JOINT SEALER			167 CONCRETE			
	KEY WORD 3			KEY WORD 4			
	189 DRAINABLE BASE			211 TRANSVERSE JOINTS			
	UNIQUE WORD			PROPRIETARY FEATURE NAME			
	233 SIX CELL SEALANT			255 DELASTIC ®SEALER			
CHRONOLOGY	Date Work Plan Approved	Date Feature Constructed:	Evaluation Scheduled Until:	Evaluation Extended Until:	Date Evaluation Terminated:		
	02-98	1999-2000	2004-2005				
	277	281	285	289	293		
QUANTITY AND COST	QUANTITY OF UNITS (ROUNDED TO WHOLE NUMBERS)		UNITS			UNIT COST (<i>Dollars, Cents</i>)	
	I-29 = 104,503, I-94 = 142,181		1 LIN. FT	5 TON	[]		
			2 SY	6 LBS			
		3 SY-IN	7 EACH				
		4 CY	8 LUMP SUM				
	297		305			306	
AVAILABLE EVALUATION REPORTS	CONSTRUCTION		PERFORMANCE		FINAL		
	315	x	x				
EVALUATION	CONSTRUCTION PROBLEMS			PERFORMANCE			
	318	1	NONE	319	1	x	EXCELLENT
		2	x SLIGHT		2		GOOD
		3	MODERATE		3		SATISFACTORY
		4	SIGNIFICANT		4		MARGINAL
		5	SEVERE		5		UNSATISFACTORY
APPLICATION	320	1	ADOPTED AS PRIMARY STD.	4	PENDING		
		2	PERMITTED ALTERNATIVE	5	REJECTED		
		3	ADOPTED CONDITIONALLY	6	NOT CONSTRUCTED		
					<i>(Explain in remarks if 3, 4, 5, or 6 is checked)</i>		
REMARKS	321 Minor problems installing sealant on I-29 and no problems occurred on the I-94 project.						

Experimental Study ND 00-01

**Evaluation Of Six-Cell
Delastic 7 Sealant**

EVALUATION REPORT

IM-8-029(026)053
and
IM-5-094(018)059

April 2004

Written By
Jeff M. Richter
Kyle Evert

Disclaimer

The contents of this report reflect the views of the author or authors who are responsible for the facts and the accuracy of the data presented herein. The contents do not reflect the official views of the North Dakota Department of Transportation or the Federal Highway Administration. This report does not constitute a standard, specification, or regulation.

TABLE OF CONTENTS

Purpose and Need	1
Objective	1
Scope	1
Location	1-2
Project Information History	2-3
Design	3-5
Construction	5
Evaluation	6-9
Summary	9
Appendix A: IM-8-029(026)053 Data	A
Appendix B: IM-5-094(018)059 Data	B
Appendix C: Product Data	C

EVALUATION OF SIX-CELL DELASTIC 7 SEALANT

Purpose and Need

This study is to evaluate the ability of **DELASTIC 7 SIX - CELL SEALANT** to remain in the joint properly. When water enters a joint, it may cause damage to the base below the concrete pavement due to freeze thaw cycles. Additionally, incompressibles may become lodged between the joints removing the space for thermal expansion. This is the rationale to use a joint sealant for concrete pavement.

Objective

The objective is to determine the effectiveness of the joint sealer to prevent debris from infiltrating the joints on concrete pavements using **DELASTIC 7 SIX - CELL SEALANT**.

Scope

The **DELASTIC 7 SIX - CELL SEALANT** was used to seal the transverse joints of project IM-8-029(026)053 and project IM-5-094(018)059. Fifteen of these joints in each project will then be evaluated annually to determine the effectiveness and durability of **DELASTIC 7 SIX - CELL SEALANT**.

Location

This experimental feature is located on Interstate 29 south of Fargo and on Interstate 94 near Dickinson. The six-cell sealant joints selected on Interstate 29 are from Reference Point 60 then proceeding 15 joints north in the northbound roadway. The six-cell sealant joints selected on Interstate 94 are from Reference Point 60 then proceeding 15 joints in east eastbound roadway. Project plan sheets and typical sections are found in Appendix A for the I-29 project and in Appendix B for the I-94 project.

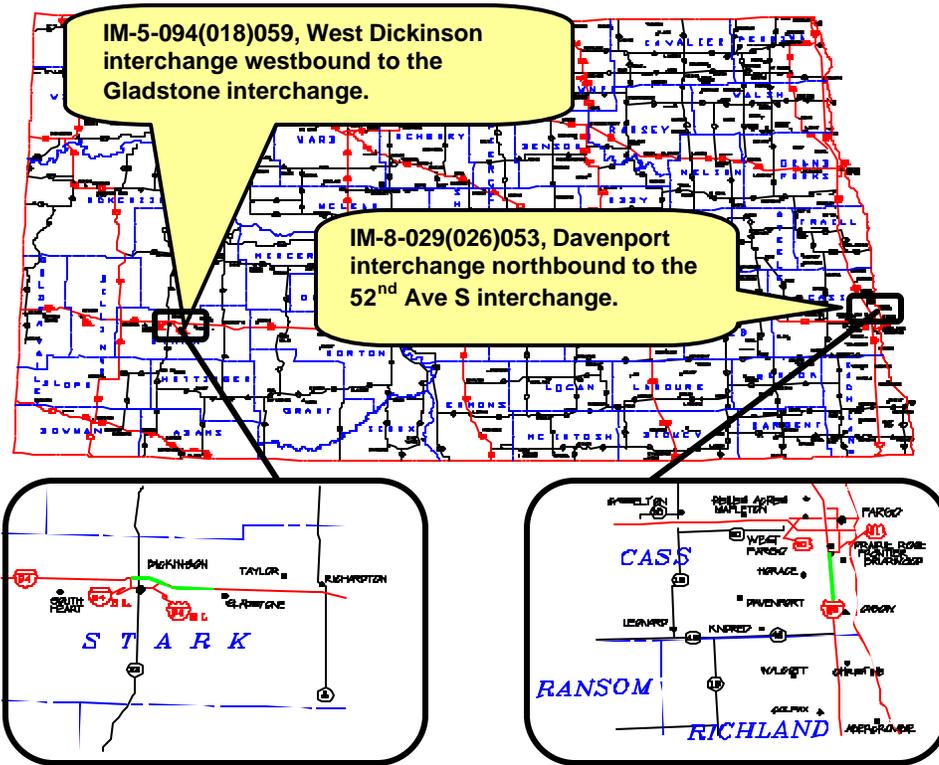


Figure 1

Project Historical Information

RIMS Data

Year	Components	Left Shoulder Width (ft)	Roadway Width (ft)	Right Shoulder Width (ft)	Depth (in)	OIL/CON Type	Class Aggregate
1999	Grade	-	59.5	-	-	-	-
1999	Salvaged Bituminous Base	-	43.0	-	8.0	-	-
1999	Permeable Cement Stabilization Base	-	28.0	-	4.0	AE	5.0
1999	Non-Reinforced PCC	4.0	24.0	10.0	10.0	AE	S4
1999	Landscaping	-	-	-	-	-	-

Table 1 – IM-8-029(026)053

RIMS Data

Year	Components	Left Shoulder Width	Roadway Width	Right Shoulder Width	Depth	OIL/CON Type	Class Aggregate
2000	Milling	-	24.0	-	-3.0	-	-
2000	Salvaged Aggregate Base	-	35.5	-	4.0	-	-
2000	Permeable Cement Stabilization Base	-	29.0	-	4.0	AE	S4
2000	Non-Reinforced PCC	-	28.0	-	9.0	AE	S3
2000	Recycled PCC	-	-	10.0	-	AE	S4
2000	Joint Space 16 Ft.	-	-	-	-	-	-
2000	Doweled	-	-	-	-	-	-
2000	Edge Drain	16.0	-	12.0	-	-	-
2002	Landscaping	-	-	-	-	-	-

Table 2 - IM-5-094(018)059

Traffic

Year	Pass>Car	Trucks	Total	Rigid ESALs – One Way
1998	7,239	1,267	8,506	844
1999	7,472	1,271	8,743	861
2000	8,008	1,401	9,409	928
2001	7,529	1,399	8,928	928
2002	8,242	1,419	9,661	945
2003	9,170	1,449	10,619	962

Table 3 – IM-8-029(026)053

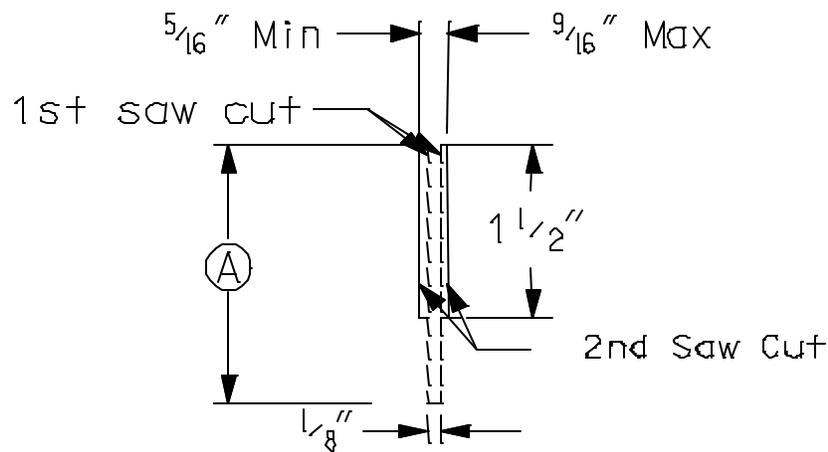
IM-5-094(018)059

Year	Pass>Car	Trucks	Total	Rigid ESALs - One Way
1999	3,816	949	4,765	634
2000	3,875	965	4,840	650
2001	4,099	1,125	5,224	635
2002	4,141	1,135	5,276	648
2003	4,112	1,125	5,237	634

Table 4 – IM-5-094(018)059

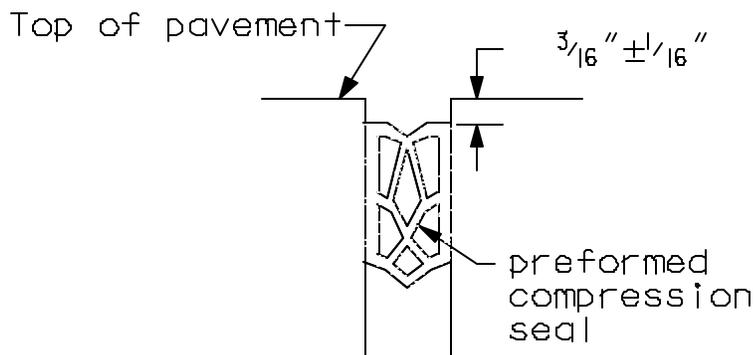
Design

The design for the use of this product on these projects is shown in Figure 2 through Figure 5. The product data sheet can be found in Appendix C. The only difference in the design between the two projects; I-29 versus I-94 is that the minimum and maximum widths of the saw cuts are different; I-29 has 5/16" Min. and 9/16" Max. while I-94 has 3/8" Min. and 7/16" Max.



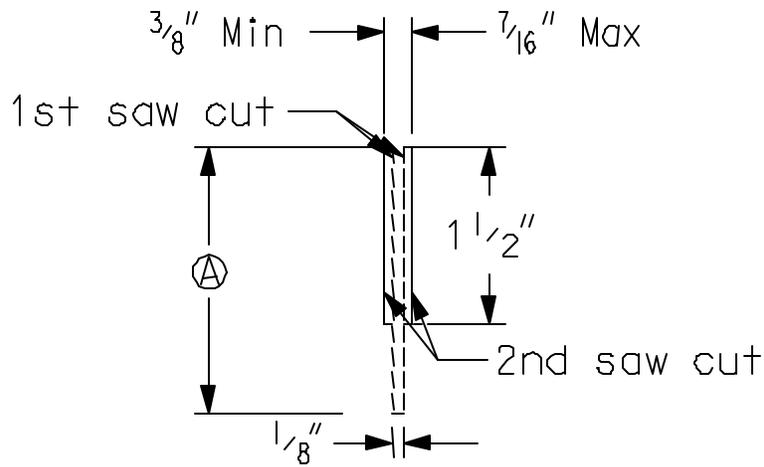
TRANSVERSE JOINT SAWING DETAIL

Figure 2 – Project IM-8-029(026)053 on Interstate 29



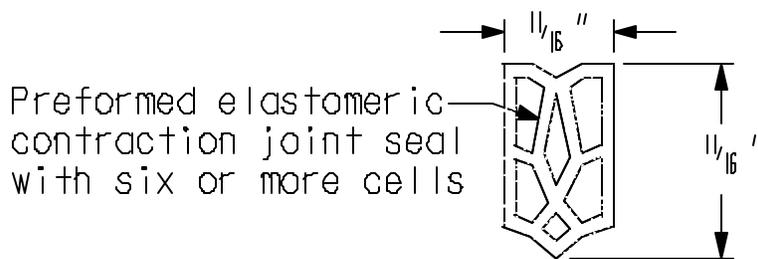
TRANSVERSE JOINT SEAL DETAIL

Figure 3 – Project IM-8-029(026)053 Interstate 29



TRANSVERSE JOINT SAWING DETAIL

Figure 4 - Interstate 94



PREFORMED CONTRACTION SEAL DETAIL

Figure 5 - Interstate 94

Construction

The construction of this research project went well. The research project was a part of projects IM-5-094(018)059 from the Dickinson District and IM-8-029(026)053 from the Fargo District. Project IM-8-029(026)053 was constructed in 1999 by "Superior Sawing" and the project engineer was Gary Heisler. Project IM-5-094(018)059 was constructed in 2000 and Ted Heinert was the project engineer. The project notes for the two projects can be found in Appendices B and C.

Evaluation

This project will be evaluated yearly for 5 years. The evaluation consists of a visual inspection of the joints to determine if the sealant has remained in place. This sealant is expected to deter the infiltration of debris into the joint.

Water and incompressibles can cause damages, such as freeze thaw and thermal expansion damages, to the roadway when infiltrated into the joint. Having a seal in good condition and operational will help prevent these damages to the roadway. The seals and joint will be evaluated for the following conditions:

- Seal twisting or departing from the joint
- Seals being depressed into the joint
- Incompressibles confined in the joint
- Spalling of the joint
- 15 joints will be used for the test section in each project

IM-8-029(026)053

There were no joints with six-cell sealant forced out of the joints in the fall 2003 evaluation. There are 3 joints with the sealant depressed into the joint. This is 3 more depressed sealants compared to the previous evaluation. The number of spalls has also increased, from 1 spall in 2002 to 3 spalls in 2003. There were no incompressibles confined in the joints. A minor spall can be seen in Photo 1. Table 5 displays the number of joints experiencing the conditions being evaluated for this research project.

IM-8-029(026)053	Year	
	2002	2003
Depressed Sealant	0	3
Twisting or Departing Sealant	0	0
Confined Incompressibles in Joint	0	0
Spalled Joints	1	3

Table 5



Photo 1 – This is a spall on a joint located on Interstate 29

IM-5-094(018)059

There were no joints with six-cell sealant forced out of the joints. The number of depressed six-cell sealants has increased from 2 in 2002 to 8 depressed six-cell sealants in 2003. The number of spalled joints has remained the same with the same spalled joint. The number of incompressibles decreased from 6 joints with incompressibles in 2002 to one in 2003. Traffic and wind have the ability to move some of the incompressibles. This may be the reason the number of joints with incompressibles in them has decreased. Table 6 displays the number of joints experiencing the conditions being evaluated for this research project.

IM-5-094(018)059	Year	
	2002	2003
Depressed Sealant	2	8
Twisting or Departing Sealant	0	0
Confined Incompressibles in Joint	6	1
Spalled Joints	1	1

Table 6

The level of depressed six-cell sealants is different for each joint. Several of the joints had the six-cell sealant depressed the entire width of the road. Other joints have only small sections that are 1' to 2' in length. Photo 2 displays a portion of a depressed sealant. A joint with incompressibles inside the joint can be seen in Photo 3. As seen in the photo the incompressibles are just resting on top of the sealant.



Photo 2 – This is a Depressed sealant in a joint on I-94.



Photo 3 – Joint with incompressibles on top of sealant.

Summary

The sealants appear to be having a problem with becoming depressed into the joint. The number of depressing sealants is increasing in both sections. The sealants on I-29 appeared to be in better condition than the I-94 sealants. The I-29 project has three joints depressed, which has increased by two since the previous year. The I-94 project has eight joints depressed, which has increased by five since the previous year.

The rest of the distresses that have been evaluated, such as spalling of joints, six-cell seals twisting, and incompressibles confined in the joint, have not changed much since the last evaluation. The number of joints on I-94 with incompressibles confined in the joint has decreased since the last evaluation. The number of spalls on the I-29 project has increased to three spalls from one spall the previous evaluation. The I-94 project does not have any new spalls. It appears the sealants are performing well except for the tendency of the sealant to depress in the joint. The sealants are only three years old and some minor problems are developing.

APPENDIX A

DESIGN DATA				
Traffic	Average Daily			Est. Max. Hr.
Current 1998	Passe: 5420	Trucks 730	Total 6150	750
Forecast 2018	Passe: 11920	Trucks 1460	Total 13380	1650
Minimum Sight Dist. for:	Design Speed 75 MPH			
Stopping 675'	Bridges			
Full Control of Access				
No Point of Access Other Than at Interchange Ramps				

JOB# 11

REGION	STATE	PROJECT NO.	SHEET NO.
8	ND	IM-8-029(026)053	1

NORTH DAKOTA DEPARTMENT OF TRANSPORTATION

FEDERAL AID PROJECT IM-8-029(026)053

PCC PAVEMENT (N Bound Rdwy)
GRADING, SURFACING & RESHAPING
OF MEDIAN CROSSOVERS, FENCING
AND REMOVAL OF RAMP CONNECTIONS

GOVERNING SPECIFICATIONS:

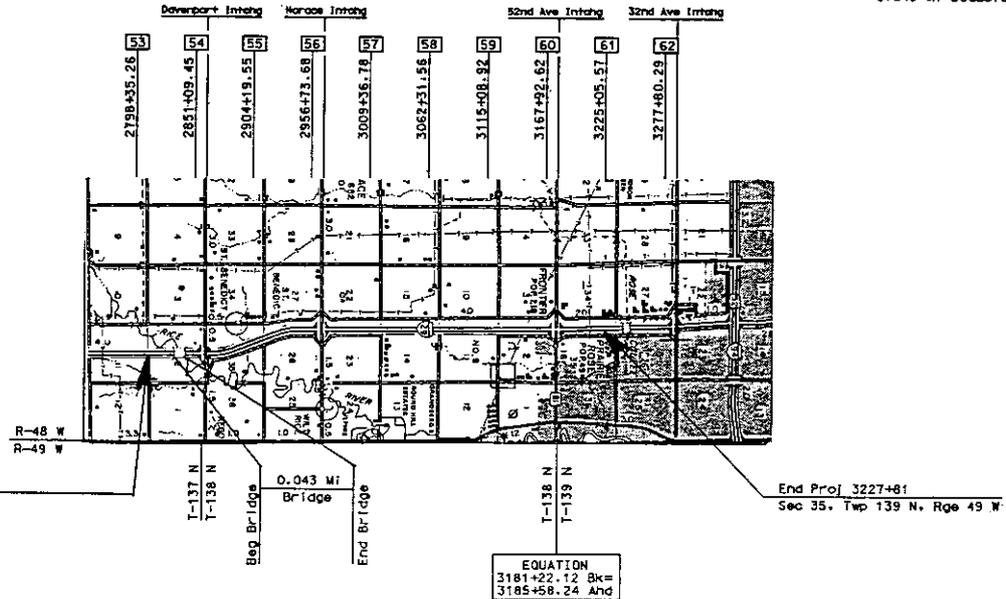
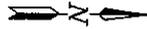
Standard Specifications adopted by the North Dakota Department of Transportation October 1997; Standard Drawings currently in effect; and other Contract Provisions submitted herein.

LENGTH OF PROJECT

Miles Gross	Miles Net
7.598	7.555

0.043 MI Deducted for Structures

In
Cass County



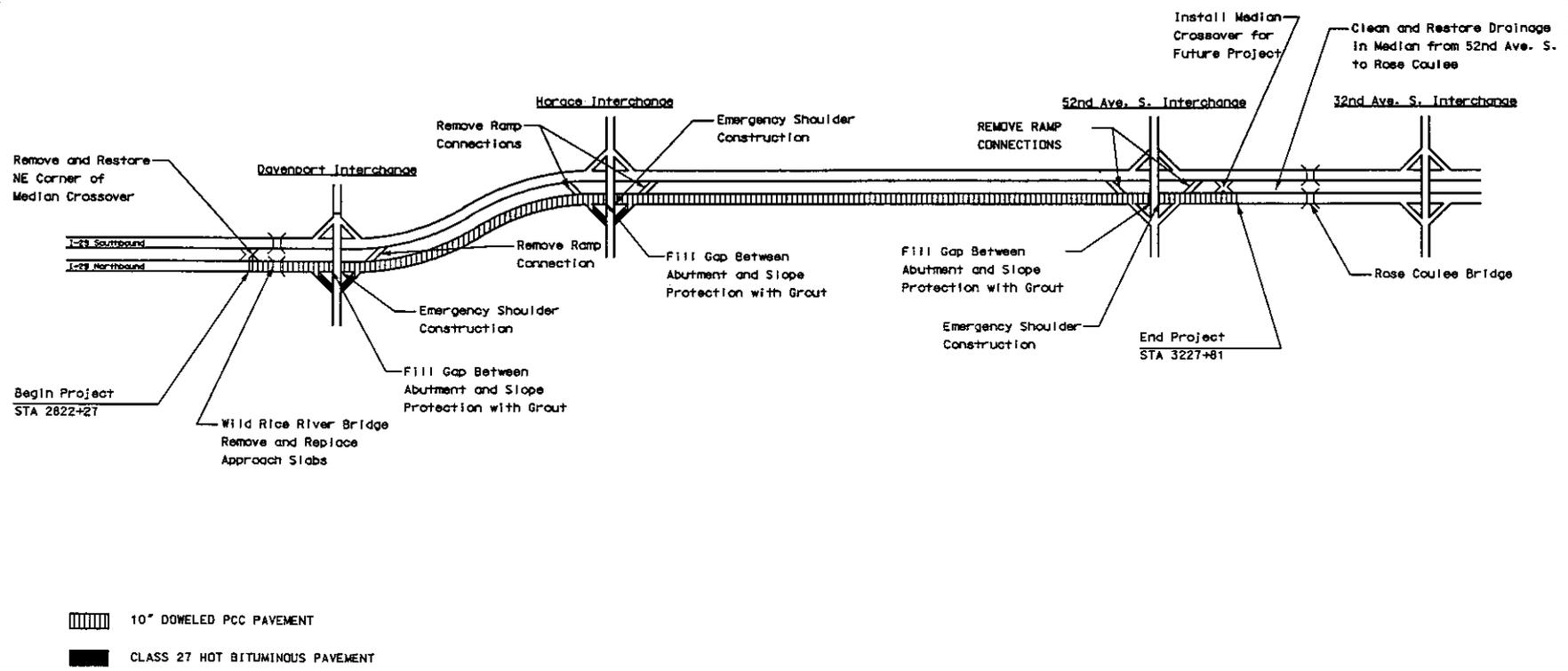
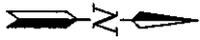
DESIGNER *Michael A. Stockley*
 DESIGNER *Harlan F. Johnson*
 DESIGNER *Harry R. Kuster*
 RECOMMEND APPROVAL *Dec. 23, 19 98*
 DESIGN ENGINEER *Dwight Sesi*

U.S. DEPARTMENT OF TRANSPORTATION
 FEDERAL HIGHWAY ADMINISTRATION
 APPROVED _____
 DIVISION ADMINISTRATOR _____ DATE _____

APPROVED DATE *12-22-98*
Dennis Jacobson
 EAST REGION ENGINEER
 NORTH DAKOTA
 DEPARTMENT OF TRANSPORTATION



ERVA REGION	STATE	PROJECT NO.	SHEET NO.
8	ND	IM-8-029(026)053	5



MAJOR SCOPE OF WORK
(Not to Scale)

FHWA REGION	STATE	FED. AID PROJ. NO.	SHEET NO.
8	N.D.	IM-8-029(026)053	9

NOTES

will be replaced with hot bituminous pavement. The remaining depth will be replaced with salvaged base course as requested or approved by the engineer.

Payment for salvaged base course will be at the unit price bid. Hot bituminous pavement patching shall include full compensation for all other labor, equipment, and materials (including asphalt cement) to complete this work.

408-P02 HOT BITUMINOUS PAVEMENT CLASS 27: The three inch or greater thickness hot bituminous pavement Class 27 shall be paver laid in two approximately equal lifts. The less than three inch hot bituminous pavement Class 27 shall be paver laid in one lift.

550-P01 TIE BARS: Tie bars shall be held in place by the metal support device securely staked to the roadbed. The device shall hold the tie bars at the correct spacing, alignment, and elevation with a 1/8-inch/foot vertical and horizontal tolerance.

550-P02 SURFACE FINISH: The carpet drag machine shall be run off a stringline.

550-P03 DOWEL BAR ASSEMBLY: Dowel bar assemblies will be included in the cost for "10 In Non-Reinforced Concrete Pavement Class AE" and "10 In Non-Reinforced Concrete Pavement Class AE - High Early".

550-P04 DOWEL BAR ASSEMBLY: After the dowel bar assembly is staked to the roadbed and the dowel bars are held firmly in place, the assembly ties running parallel to the dowel bars shall be removed to allow for free movement of the dowel bars.

550-P05 GAPS IN MAINLINE PAVING: The interchange ramp connections will result in gaps in the contractor's mainline paving operation. These gaps shall be constructed after concrete paving is completed either side of the gaps. Concrete placed in the gap areas shall be a high early mix. All costs for the high early concrete in gap areas shall be paid for as "10 Inch Non-Reinforced Concrete Pavement Cl AE - High Early." Traffic may be allowed on the gap concrete after 4 days or after the concrete has attained a flexural strength

of at least 500 psi or a compressive strength of 3000 psi. All joint sealing shall be completed before traffic is allowed on the new gap concrete. A construction joint shall be placed at each end of the gap paving area and the cost for these shall be included in the unit price bid for "10-Inch Non-Reinforced Concrete Pavement Cl AE."

550-P06 INTERCHANGE RAMP CONNECTIONS: During the period that the gap in the mainline pavement (as a result of the interchange ramp connections) is being paved, the public traffic using the ramps shall be routed around the gap paving area with temporary ramp connections. The temporary ramp connections shall be constructed in accordance with Section 710. The ramp pavement removal, reshaping, subgrade preparation, aggregate placement, and paving operation shall also be performed during the period that the mainline gap is being paved. Ramp traffic shall be maintained during this period. All costs for flagging, signing, aggregate, and water necessary for constructing and maintaining the ramp connection shall be paid for at the unit price bid for each item.

550-P07 TRANSVERSE JOINTS SAWING AND SEALING: The contractor shall be allowed to skip saw the ten-foot PCC shoulder when it is paved separately. No other skip sawing shall be allowed.

550-P08 REINFORCING OVER PIPES: Additional reinforcing steel shall be provided and placed at the locations shown on the "Pavement Reinforcement Detail." All costs to provide, place, and support the reinforcing steel as shown shall be included in the price bid for "10-Inch Non-Reinforced PCC Pavement Cl AE."

550-P09 REMOVAL OF CONTINUOUS REINFORCED CONCRETE PAVEMENT: This pay item shall consist of the mainline continuous concrete pavement, and 20' of the 40' approach slabs at the Wild Rice River bridge. Existing bituminous blowup repairs, sealing material, and pothole patching removal is included in the cost of "Removal of Continuous Reinforced Concrete Pavement."

550-P10 ADDITIONAL PAVEMENT IMPRINTING: After texturing, the milepoint numbers shall be imprinted into the concrete

ESTIMATE OF QUANTITIES

FHWA REGION	STATE	FED. AID PROJ. NO.	SHEET NO.
8	N.D.	IM-8-029 (026) 053	18

SPEC CODE	ITEM DESCRIPTION	UNIT	MAINLINE	TOTAL
103	0100 CONTRACT BOND	L SUM	1	1
202	0104 REMOVAL OF STRUCTURE	EA	2	2
202	0138 REMOVE & SALVAGE BASE COURSE	TON	24,093	24,093
203	0103 COMMON EXCAVATION-TYPE C	CY	8,572	8,572
203	0138 COMMON EXCAVATION-SUBCUT	CY	9,810	9,810
203	0208 GUARDRAIL EMBANKMENT-TYPE C	EA	2	2
216	0100 WATER	M GAL	3,904	3,904
230	0104 RESHAPING CONNECTION	EA	1	1
230	0106 RESHAPING ROADWAY	MILE	8.227	8.227
230	0190 SUBGRADE PREPARATION-TYPE C-12IN	MILE	7.555	7.555
302	0100 SALVAGED BASE COURSE	TON	69,301	69,301
302	0107 AGGREGATE FOR SUBGRADE REINFORCEMENT	TON	18,394	18,394
302	0120 AGGREGATE BASE COURSE CL 5	TON	40,149	40,149
304	5000 PERMEABLE STABILIZED BASE COURSE-SALVAGED	SY	128,534	128,534
401	0100 MC70 OR 250 LIQUID ASPHALT	GAL	32,134	32,134
401	0152 SS1H OR CSS1H EMULSIFIED ASPHALT	GAL	4,570	4,570
401	0160 BLOTTER MATERIAL CL 44	TON	121	121
405	0110 REMOVE & SALVAGE BITUMINOUS SURFACING	TON	9,307	9,307
408	0176 HOT BITUMINOUS PAVEMENT CL 27	TON	2,663	2,663
408	0198 HOT BITUMINOUS PAVEMENT PATCHING	TON	500	500
409	0445 PG 58-28 ASPHALT CEMENT	TON	160	160
410	0105 MILLING BITUMINOUS PAVEMENT	SY	222	222
550	0112 8IN NON-REINF CONCRETE PAVEMENT CL AE	SY	5,009	5,009
550	0118 10IN NON-REINF CONCRETE PAVEMENT CL AE	SY	173,550	173,550
550	0178 10IN NON-REINF CONCRETE PAVEMENT CL AE-HIGH EARLY	SY	8,000	8,000
550	0217 BRIDGE APPROACH SLAB-REMOVE & REPLACE	SY	182	182
550	0809 PREFORMED ELASTOMERIC COMPRESS JT SEAL 9/16IN	LF	104,503	104,503
550	0958 LONGITUDINAL JOINT SILICONE SEAL	LF	3,711	3,711
560	1580 PREPARE STOCKPILE SITE	L SUM	1	1
560	1590 REMOVAL OF CONCRETE PAVEMENT	SY	7,430	7,430
560	1591 REMOVAL OF CONTINUOUS REINF CONCRETE PVMT	SY	168,344	168,344
602	1210 BRIDGE END POST MODIFICATION	EA	2	2
702	0100 MOBILIZATION	L SUM	1	1
704	0100 FLAGGING	MHR	3,000	3,000

APPENDIX B

DESIGN DATA				
Traffic	Average Daily			Max. Hr.
Current 1998	Pass: 1360-2026	Trucks 460-510	Total 1820-2536	230-320
Forecast 2019	Pass: 1820-2900	Trucks 780-900	Total 2600-3800	325-480
Minimum Sight Dist. for:		Design Speed		
Stopping 125 FT		Bridges		
Full Control of Access				
No Point of Access Other Than at Interchange Ramps				

JOB# 12

FED. REGION	STATE	PROJECT NO.	SHEET NO.
8	ND	IM-5-094(018)059	1

NORTH DAKOTA DEPARTMENT OF TRANSPORTATION

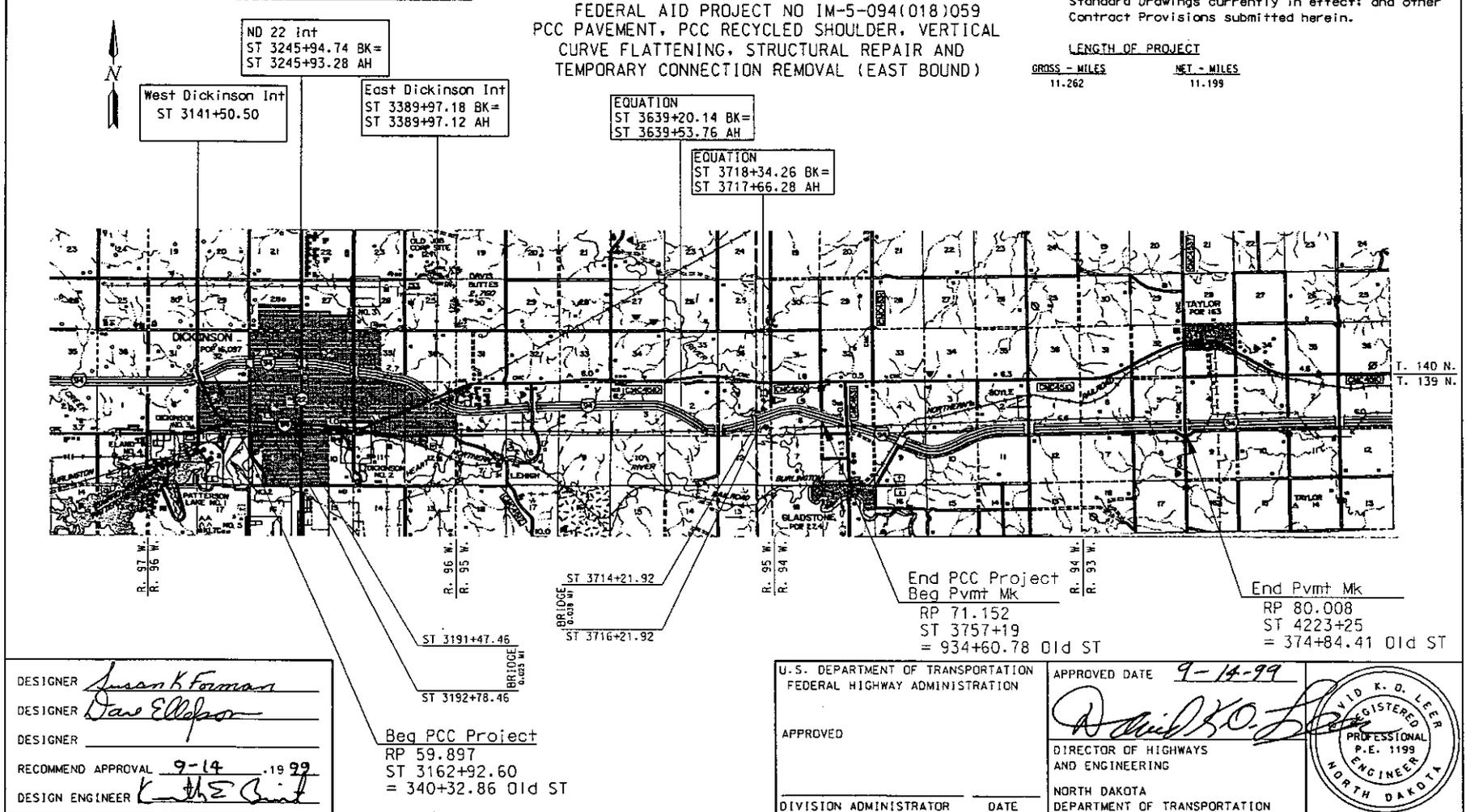
GOVERNING SPECIFICATIONS:

Standard Specifications adopted by the North Dakota Department of Transportation October 1997: Standard Drawings currently in effect; and other Contract Provisions submitted herein.

IN STARK COUNTY
FEDERAL AID PROJECT NO IM-5-094(018)059
PCC PAVEMENT, PCC RECYCLED SHOULDER, VERTICAL
CURVE FLATTENING, STRUCTURAL REPAIR AND
TEMPORARY CONNECTION REMOVAL (EAST BOUND)

LENGTH OF PROJECT

GROSS - MILES NET - MILES
11.262 11.199



DESIGNER *Susan K. Forman*
 DESIGNER *Dave Ellsper*
 DESIGNER _____
 RECOMMEND APPROVAL 9-14 .19 99
 DESIGN ENGINEER *Keith E. Smith*

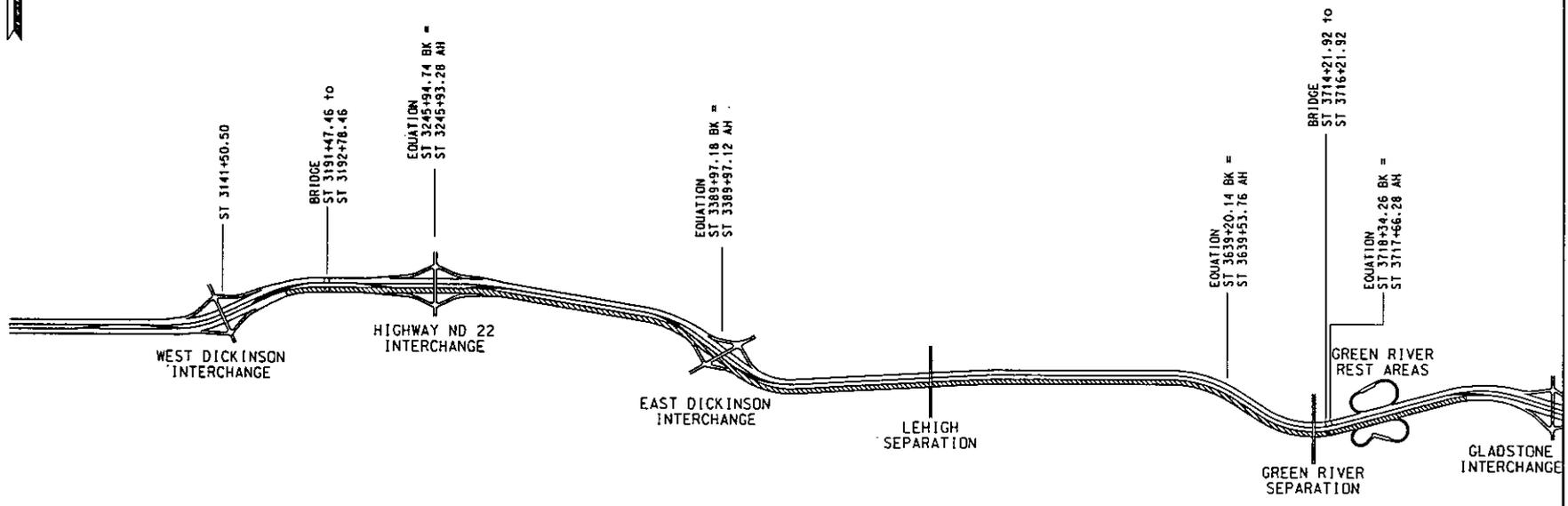
Req PCC Project
 RP 59.897
 ST 3162+92.60
 = 340+32.86 Old ST

U.S. DEPARTMENT OF TRANSPORTATION
 FEDERAL HIGHWAY ADMINISTRATION
 APPROVED _____
 DIVISION ADMINISTRATOR DATE

APPROVED DATE 9-14-99
David K. O. Leer
 DIRECTOR OF HIGHWAYS
 AND ENGINEERING
 NORTH DAKOTA
 DEPARTMENT OF TRANSPORTATION



FHWA REGION	STATE	PROJECT NO.	SHEET NO.
8	ND	IM-5-094(018)059	3



LEGEND

- Remove and Reconstruct Main Line Pavement
- Remove Once Traffic Returns to Normal Operation.

SCOPE OF WORK
 W DICKINSON INTERCHANGE TO
 NEAR GLADSTONE (EB)
 PCC AND RPCC PAVEMENT

FHWA REGION	STATE	FED. AID PROJ. NO.	SHEET NO.
8	N.D.	IM-5-094 (018) 059	5

NOTES

	may be obtained within the right of way at areas determined by the engineer.	409-P01	HOT BITUMINOUS PAVEMENT: All overlay areas shall be applied in two lifts.
203-P04	Topsoil removal at the east Dickinson crossroad widening will not be bid separately but will be included in the price bid for "Borrow."	410-P01	MILLING BITUMINOUS PAVEMENT ON MAINLINE: This work shall consist of milling approximately 3-4 inches of bituminous pavement, an asphalt-vulcanized rubber membrane, and 1/4 inch minimum of concrete surface. This material shall all be milled in one operation on the mainline. If the milling is done before the shoulders are removed, the bituminous shoulders shall be milled approximately the same depth or removed to prevent ponding of water. All cost for milling, loading, hauling, and stockpiling of the material shall be included in the unit price bid for "Milling Bituminous Pavement," by the ton.
203-P05	DITCH BLOCKS: Ditch block slope flattening will not be bid separately but will be included in the price bid for "Roadway Reshaping."		
203-P06	BORROW: All borrow is to be contractor furnished borrow.		
230-P01	The ramp taper reshaping shall not be measured and paid for separately but will be included in the price bid for mainline reshaping roadway. The mainline and ramp reshaping shall be performed prior to the subgrade preparation.	410-P02	MILLING BITUMINOUS PAVEMENT ON RAMPS AND CROSSROADS: The ramp and crossroad hot bituminous pavement overlay shall begin after the mainline and ramp PCC paving has been completed. The ramp and crossroad milling shall begin no more than two weeks prior to the hot bituminous pavement overlay. The milled material shall become the property of the state of North Dakota and shall be delivered to the Dickinson district yard. All cost for milling, loading, hauling, and stockpiling of the material shall be included in the unit price bid for "Milling Bituminous Pavement," by the square yard at plan quantity.
302-P01	TRIMMING SALVAGED BASE COURSE: Use surface tolerance Type B for the salvaged base course. Excess material removed from high points of the salvaged base course by the trimming operation shall be reincorporated into the salvaged base course. The cost for providing the required grade and cross section shall be included in the unit price bid for "Salvaged Base Course."		
302-P02	SALVAGED BASE COURSE: The salvaged base course shall consist of a uniform blend of salvaged PCC fines, salvaged bituminous, and salvaged aggregate material that is belt or bin blended. There will be a maximum of 20 percent salvaged PCC fines in the blend.	550-P01	WATER-REDUCING ADMIXTURE: Type A water-reducing concrete admixture shall be used in the 28' mainline concrete mix on this project. Dosage shall be at the manufacturer's recommendation.
302-P03	TRIMMING AND PRIME: Keep the trimming of the salvaged base course within one mile of the completed laydown of this course. Keep the application of the prime coat within one mile of the trimmed, accepted salvaged base course.	550-P02	SURFACE FINISH: The carpet drag machine shall be run off a stringline.
		550-P03	CONTRACTION JOINTS: Transverse joints shall be sealed using a preformed elastomeric compression joint seal.
304-P01	PERMEABLE STABILIZED BASE: Portland Cement shall be used as the stabilizing agent. The aggregate for the bid item "Permeable Stabilized Base Course - Salvaged" shall be produced from the salvaged concrete pavement.	550-P04	ADDITIONAL PAVEMENT IMPRINTING: After texturing, the milepoint numbers shall be imprinted into the 10' shoulder concrete surface by the contractor about one foot from the 12' driving lane, so the numbers can be read in the direction of the traffic. A zero shall be imprinted into the concrete at all headwall locations.
401-P01	BLOTTER: Class 44 blotter shall not be measured and paid for separately but will be included in the unit price bid for "MC 70 or 250 Liquid Asphalt."		

FHWA REGION	STATE	FED. AID PROJ. NDL	SHEET NO.
8	N.D.	IM-5-094 (018) 059	6

NOTES

- 550-P05 GAPS IN MAINLINE PAVING: The interchange ramp connections will result in gaps in the contractor's mainline paving operation. These gaps shall be constructed after concrete paving is completed either side of the gaps. Concrete placed in the gap areas shall be a 7.2 bag mix without flyash. All costs for the concrete in gap areas shall be paid for at the same unit price as other mainline concrete. Traffic may be allowed on the gap concrete after four days or after the concrete has attained a flexural strength of at least 500 psi or a compressive strength of 3,000 psi. All joint sealing shall be completed before traffic is allowed on the new gap concrete. A construction joint shall be placed at each end of the gap paving area and the cost for these shall be included in the unit price bid for "9-Inch Non Reinforced Concrete Pavement CL AE."
- 550-P06 Flyash and cement will not be bid separately but will be included in the price bid for "9 In Non-Reinforced Concrete Pavement CL AE."
- 550-P07 TIE BARS: Supplemental Specification 550.04 H shall be modified on this project to allow the tie bars to be inserted in the plastic concrete by approved mechanical devices after the concrete has been spread, struck-off, and consolidated to full depth.
- 550-P08 CORED SAMPLE: The contractor shall be responsible for taking cored samples. The samples shall be taken according to specification 550.04Q.3. The samples shall be paid as "Cored Sample EA."
- 550-P09 The longitudinal construction joints must be supported by a metal keyway support.
- 550-P10 TESTS: If two consecutive tests fail in the concrete permeable base aggregate, edge drain aggregate, or the mainline concrete aggregate, operations shall be halted till the contractor has proven to the engineer that future samples will be acceptable.
- 560-P01 TRANSVERSE JOINTS SAWING AND SEALING: The contractor shall be allowed to skip saw the ten-foot PCC shoulder when it is paved separately. No other skip sawing shall be allowed.
- 560-P02 EXISTING REINFORCING STEEL: The reinforcing removed from the existing PCC Pavement shall become the property of the contractor and shall be removed from the highway right of way upon removal from the concrete.
- 560-P03 REMOVAL OF CONCRETE PAVEMENT: Equipment that might damage the subgrade will not be allowed to load the existing concrete as it is removed from the roadway.
- 560-P04 INTERCHANGE RAMP CONNECTION DETOURS: During the period that the gap in the mainline pavement (as a result of the interchange ramp connections is being paved and the edgedrains installed, the public traffic using the ramps shall be routed around the gap paving area with the ramp connection detours. The ramp pavement removal, reshaping, subgrade preparation, and paving operation shall also be performed during the period that the mainline gap is being paved and ramp traffic shall be maintained during this period. All costs for materials necessary for constructing and maintaining the interchange ramp connection detours shall be measured and paid for at the unit price bid for each ramp connection detour as "Temporary Bypass."
- 560-P05 CONCRETE PAVEMENT REMOVAL: At the completion of the project, any excess concrete material (from concrete removal) shall become the property of the contractor.
- 560-P06 Flyash and cement will not be bid separately but will be included in the price bid for "9 In Non-Reinforced Recycled Concrete Pavement CL AE."
- 602-P01 JERSEY BARRIER FORMED OR SLIPFORMED: This item shall consist of constructing reinforced concrete jersey barriers at median bridge piers as shown in the plans. Where barriers are cast on both sides of bridge piers, the space between the two barrier walls shall be filled with aggregate and capped with a 4" reinforced concrete slab as shown in the plans. Each barrier wall shall not be measured separately. The pay length shall be from the beginning to the end of the complete barrier wall installation. The price bid for the item "Jersey Barrier Formed or Slipformed" shall include furnishing and installing Class AAE-3 concrete, Grade 60 reinforcing steel, mastic, joint sealant materials, aggregate, and all other materials, equipment, and labor required to complete the installation as shown in the plans.

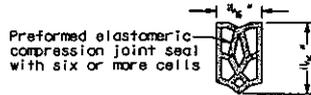
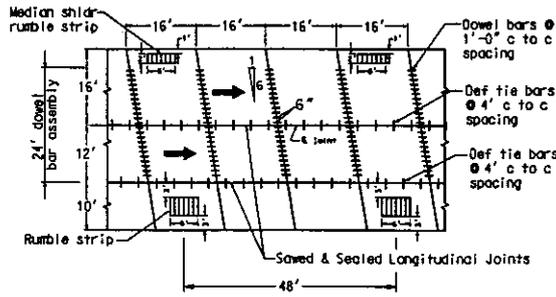
ESTIMATE OF QUANTITIES

FHWA REGION	STATE	FED. AID PROJ. NO.	SHEET NO.
8	N.D.	IM-5-094(018)059	15

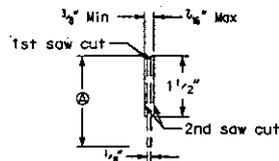
SPEC CODE	ITEM DESCRIPTION	UNIT	PROJECT	TOTAL
103	0100 CONTRACT BOND	L SUM	1	1
202	0115 REMOVAL OF CONCRETE-SITE 1	L SUM	1	1
202	0117 REMOVAL OF CONCRETE-SITE 3	L SUM	1	1
202	0118 REMOVAL OF CONCRETE-SITE 4	L SUM	1	1
202	0119 SAW CONCRETE	LF	832	832
202	0121 REMOVE & SALVAGE BITUMINOUS SURFACING	TON	89,438	89,438
202	0130 REMOVAL OF CURB & GUTTER	LF	320	320
203	0137 COMMON EXCAVATION-SUBGRADE REPAIR	CY	29,166	29,166
203	0140 BORROW	CY	2,051	2,051
203	0196 EMBANKMENT-TYPE B	EA	6	6
203	0208 GUARDRAIL EMBANKMENT-TYPE C	EA	11	11
210	0101 CLASS I EXCAVATION	L SUM	1	1
210	0203 FOUNDATION PREPARATION-SITE 2	L SUM	1	1
210	0204 FOUNDATION PREPARATION-SITE 3	L SUM	1	1
216	0100 WATER	M GAL	5,924	5,924
230	0106 RESHAPING ROADWAY	MILE	11.199	11.199
230	0182 SUBGRADE PREPARATION-TYPE B-12IN	MILE	4.769	4.769
230	0186 SUBGRADE PREPARATION-TYPE B-18IN	MILE	2	2
230	0188 SUBGRADE PREPARATION-TYPE B-24IN	MILE	2	2
302	0100 SALVAGED BASE COURSE	TON	132,954	132,954
302	0113 AGGREGATE BASE COURSE CL 3	TON	4,410	4,410
304	5000 PERMEABLE STABILIZED BASE COURSE-SALVAGED	SY	190,276	190,276
401	0100 MC70 OR 250 LIQUID ASPHALT	GAL	53,752	53,752
401	0150 SSIH OR CSSIH OR MSI EMULSFIED ASPHALT	GAL	5,651	5,651
408	0185 HOT BITUMINOUS PAVEMENT CL 29	TON	12,271	12,271
408	0445 PG 58-28 ASPHALT CEMENT	TON	979	979
410	0100 MILLING BITUMINOUS PAVEMENT	TON	22,721	22,721
410	0105 MILLING BITUMINOUS PAVEMENT	SY	52,723	52,723
550	0117 9IN NON-REINF CONCRETE PAVEMENT CL AB	SY	189,345	189,345
550	0215 CONCRETE BRIDGE APPROACH SLAB	SY	338.6	338.6
550	0217 BRIDGE APPROACH SLAB-REMOVE & REPLACE	SY	359.5	359.5
550	0240 DOWELED CONTRACTION JOINT ASSEMBLY	LF	89,799	89,799
550	0810 PREFORMED ELASTOMERIC COMPRESS JT SEAL 11/16IN	LF	142,181	142,181
550	0958 LONGITUDINAL JOINT SILICONE SEAL	LF	8,198	8,198

ESTIMATE NUMBER: 1967 RUN DATE: 09/17/1999 TIME: 10:44:15

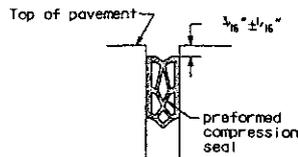
Revised November 1, 1999



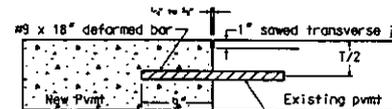
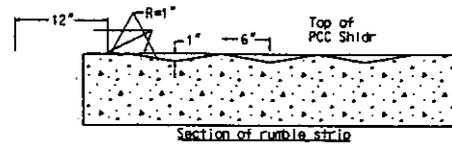
PREFORMED COMPRESSION SEAL DETAIL



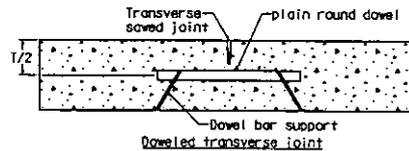
TRANSVERSE JOINT SAWING DETAIL



TRANSVERSE JOINT SEAL DETAIL



Deformed bar installation (2'-0" c to c)



Longitudinal joint tie bars @ 4'-0" spacing

	PAVEMENT DEPTH	
	8"	9"
10' shldr	#5 x 2'-6"	
CI ml	#5 x 3'-0"	

Dowel bars shall be 1 1/2" x 18" when T = 10" or less and shall be 1 1/2" x 18" when T is greater than 10".

⊙ = one-third thickness of the pcc pavement.

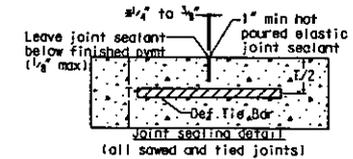
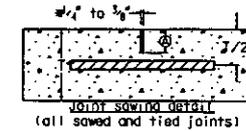
⊙ = Omit rumble strips through ramp taper areas.

T = pavement depth

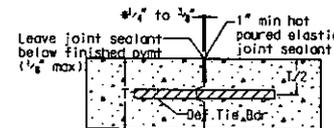
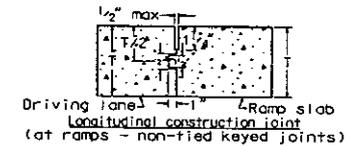
#Width requirement for top 1" only: Bottom portion of saw cut may be narrower.

72 hours advance notice must be given to the project engineer and to Clayton Schumaker (701)328-6906, before installation of the preformed compression seal.

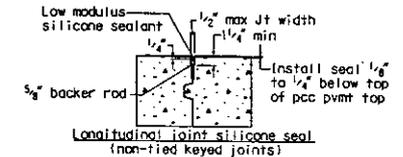
A manufacturers representative of the preformed compression seal must be on site during its installation.



Longitudinal construction joint (tied keyed joints)
(keyway and joint dimensions are the same as non-tied keyed joints)



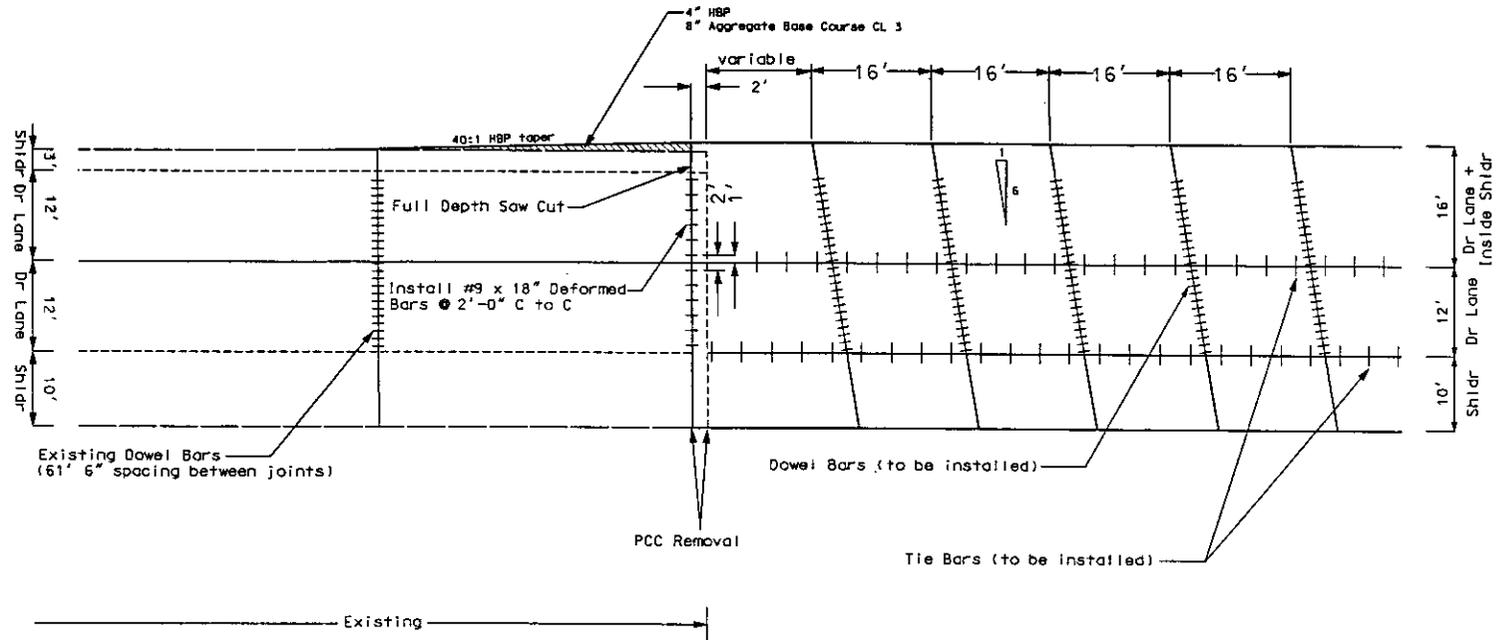
Longitudinal construction joint hot pour elastic joint seal (tied keyed joints)



Longitudinal joint silicone seal (non-tied keyed joints)

JOINT DETAILS
DOWELED 38' PCC PAVEMENT

NOTE: Full depth saw cut shall be incidental to removal of concrete pavement.

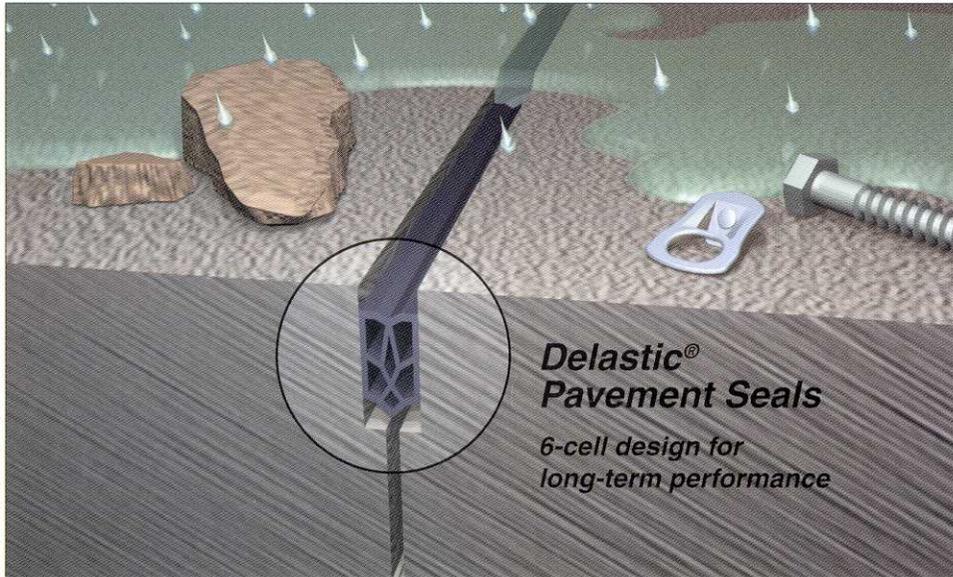


ESTIMATED QUANTITIES			
DESCRIPTION	UNIT	MAINLINE	3'+10' SHOULDER
Removal of Concrete Pvmt (9" JRCP, 24' Wide)	SY	5	
Remove and Salvage Bituminous Surfacing (32.6492 SF and 1.875 TON/CY)	TON		5
Hot Bituminous Pavement CL 29 (29.7415 SF and 2 TON/CY)	TON		19
PG 58-28 Asphalt Cement	TON		1
Aggregate Base Course CL 3 (29.7415 SF and 1.875 TON/CY)	TON		37

JOINT AT WEST END OF PROJECT

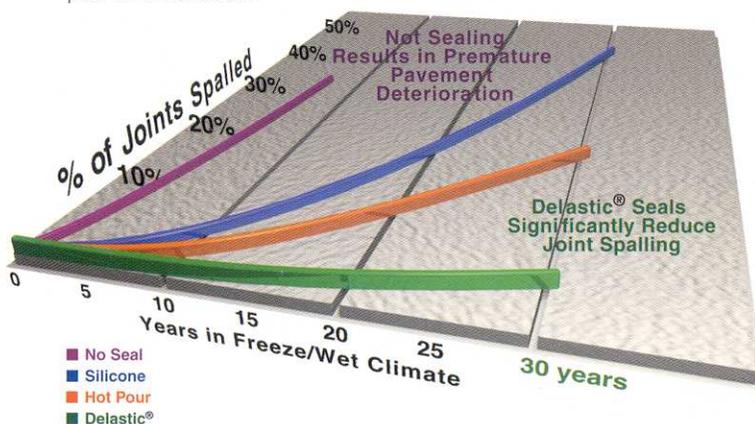
APPENDIX C

Why be short-sighted when your pavement must perform long-term?



Admit it. You specify concrete pavement because it performs. So why compromise that decision by using cheap, ineffective sealants... or worse, choosing not to seal the joints at all?

For nearly 40 years, D.O.T.s and other pavement engineers have used Delastic® Pavement Seals to improve pavement performance while reducing overall pavement costs.



Based on an independent study showing the effects of joint sealing on extending PCC pavement performance.

It's a fact. When compared to not sealing at all or using ineffective sealants, Delastic® Pavement Seals dramatically extend the life of your concrete pavement.

So, when you design your concrete pavement, think long-term. Specify Delastic® Pavement Seals.

Protect Your Investment



D.S. BROWN

The D.S. Brown Company
300 E. Cherry Street
North Baltimore, OH 45872

419.257.3561
fax: 419.257.2200
e-mail: dsb@dsbrown.com
www.dsbrown.com

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MEETING THE CHALLENGE

Building highways and airport runways to last is a challenging engineering assignment. Especially when the pavement must withstand heavy traffic, weather extremes, debilitating fuels and de-icing chemicals. You need reliable products with a proven track record. At The D.S. Brown Company, we can help.

Our Delastic® Pavement Seals have been specified and installed on highways and airport runways since 1960. Through time, they have proven to be durable and cost-effective.

But sealing joints are only one aspect of dealing with pavement design. You must also consider the repair of damaged concrete as well as methods to upgrade existing airfield lighting. Our unique Delpatch™ Elastomeric Concrete has been designed to solve those difficult problems.

This brochure provides details about our Delastic® Pavement Seals and Delpatch™ Elastomeric Concrete. For more information, contact our Pavement Products Sales Department in our corporate office at 419.257.3561.

DELASTIC® PAVEMENT SEALS

Why Delastic® seals?

Our Delastic® Pavement Seals for concrete pavement have one main purpose – to prevent water and debris from entering the joint. If that happens, the pavement can crack, chip, buckle and prematurely deteriorate.

Delastic® Pavement Seals are extruded from compounds of neoprene (polychloroprene) which meet or exceed current ASTM standard specifications. They are compressed and remain in contact with the joint walls while

allowing the concrete pavement to expand and contract during temperature changes.

There are other joint seal products available, but each has its disadvantages. Silicone caulk can fail because of poor adhesion to the concrete walls. Hot pour asphalt systems do not age well and require the pavement joints to be resealed often, significantly increasing the overall cost. Only Delastic® Pavement Seals have proven to remain durable and effective through the years.

MEETING SPECIFICATIONS

Delastic® seals meet ASTM standard specifications. They are also recognized by the FHWA, U.S. Army Corps of Engineers, the U.S. Air Force, consulting

engineers and other agencies as an effective, long-lasting concrete pavement joint seal solution.

Easy-to-install Delastic® Pavement Seals are an excellent choice for high-traffic airport runways, taxiways and aprons.



ABOUT THE D.S. BROWN CO.

SINCE 1960, THE D.S. BROWN COMPANY HAS BEEN A WORLDWIDE SUPPLIER OF ENGINEERED PRODUCTS TO THE HIGHWAY AND BRIDGE CONSTRUCTION INDUSTRIES. WE ARE A FULLY-INTEGRATED MANUFACTURER, PERFORMING AND CONTROLLING THE FOLLOWING FUNCTIONS IN OUR FACILITIES:

- RESEARCH AND DEVELOPMENT
- ENGINEERING/CAD DESIGN
- DYNAMIC LOAD TESTING
- COMPOUNDING, RUBBER MIXING, EXTRUDING AND MOLDING
- CUSTOM STEEL FABRICATION AND MACHINING

D.S. BROWN OFFERS ITS CUSTOMERS DECADES OF EXPERIENCE ALONG WITH FULLY-INTEGRATED ENGINEERING AND MANUFACTURING AS WELL AS KNOWLEDGEABLE SALES PROFESSIONALS, ALL OF WHICH HAVE ENABLED THE D.S. BROWN COMPANY TO BECOME THE LEADING MANUFACTURER AND SUPPLIER TO THE HIGHWAY AND BRIDGE CONSTRUCTION MARKET.

DELASTIC® ADVANTAGES

Our “E” and “V” series Delastic® seals are the primary sealing system for concrete pavement slabs in highways, airport aprons and runways. Delastic® seals offer these advantages:

- Ability to seal joints in concrete even when moisture is present
- Resistance to jet fuel and other chemicals
- Speed of installation
- Product cleanliness
- Ease of inspection
- Dramatic reduction in concrete joint spalling

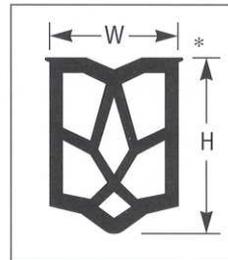
Delastic® Seal Catalog No.	SEAL CHARACTERISTICS			JOINT DESIGN CRITERIA		
	Nominal Width (W)	Nominal Height (H)	Max. Movement ¹	Narrowest Opening ² (A)	Widest Opening ³ (A)	Minimum Depth (B)
E-437	0.437 (11.11)	0.937 (23.81)	0.184 (4.67)	0.187 (4.75)	0.371 (9.42)	1.250 (31.75)
V-562	0.562 (14.29)	0.625 (15.88)	0.178 (4.52)	0.250 (6.35)	0.478 (12.14)	1.125 (28.58)
E-686	0.687 (17.46)	0.687 (17.46)	0.259 (6.58)	0.325 (8.26)	0.584 (14.83)	1.250 (31.75)
E-816	0.812 (20.64)	0.812 (20.64)	0.348 (8.84)	0.350 (8.89)	0.698 (17.73)	1.500 (38.10)
E-1006	1 (25.40)	1 (25.40)	0.450 (11.43)	0.400 (10.16)	0.850 (21.59)	1.750 (44.45)
E-1256	1.250 (31.75)	1 (25.40)	0.612 (15.54)	0.450 (11.43)	1.062 (26.97)	2 (50.80)
V-1625	1.625 (41.28)	1.125 (28.58)	0.781 (19.84)	0.600 (15.24)	1.381 (35.08)	2.375 (60.33)
E-2000	2 (50.80)	1.500 (38.10)	0.950 (24.13)	0.750 (19.05)	1.700 (43.18)	2.625 (66.68)
E-2500	2.500 (63.50)	2.500 (63.50)	1.100 (27.94)	0.775 (19.69)	2.125 (53.98)	4 (101.60)
E-3000	3 (76.20)	2.500 (63.50)	1.260 (32.00)	1.200 (30.48)	2.550 (64.77)	4.250 (107.95)

First number shown in bold represents inches, metric dimensions (MM) are shown in parentheses.

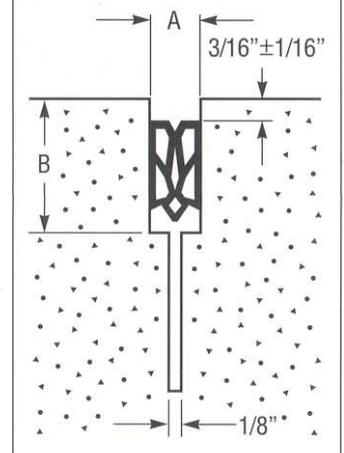
Notes:

- * Thickness of the seal wall and internal web are not drawn to scale.
- 1 Maximum movement which seal will accommodate in joint with correct design.
- 2 A narrower opening will place excessive stress on the seal and may cause premature failure.
- 3 A wider opening may not provide sufficient compressive force to hold the seal in place.

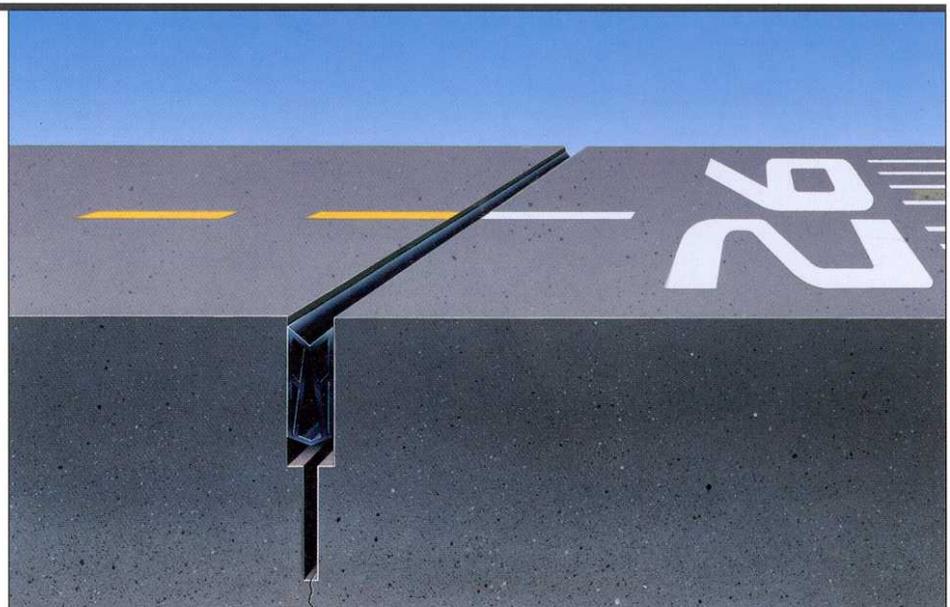
Meets ASTM D 2628 and AASHTO M-220 standard specifications.



Typical joint design for the “E” and “V” series pavement seals.



Delastic® seals effectively protect concrete pavement in highways, airport aprons and airport runways by keeping moisture, debris and incompressible material out of the joint opening.



DELASTIC® INSTALLATION

In all instances, the joint walls must be reasonably clean and free of spalls with a properly designed width. To facilitate seal installation, D.S. Brown offers its Delastall™ 105 and SealTek™ installation machines for rental or purchase.

LUBRICANTS

Delastilube™ and Delastilube™-HS are used during installation to lubricate the seal when it is inserted in the joint opening. The minimum installation temperature on job sites should be 40°F (4°C) to assure proper performance of the lubricant.

MATERIAL/SPECIFICATIONS

For “E” and “V” series Delastic® seals, ASTM standard specification D 2628-97 applies. Delastilube™ lubricant meets ASTM D 2835 requirements while Delastilube™-HS meets ASTM D 4070.

PACKAGING

Delastic® seals are shipped on reels, spools or in boxes marked in ordered lengths according to customer

requirements. Delastilube™ lubricant is supplied in one, five or 55 gallon containers.

ASTM D 2628 DELASTIC® PAVEMENT SEALS PHYSICAL REQUIREMENTS		
Properties	Requirements	ASTM Test Method
Tensile strength, min, psi (MPa)	2000 (13.8)	D 412
Elongation at break, min, %	250	D 412
Hardness, Type A durometer, points	55±5	D 2240 (modified) ^A
Oven aging, 70 h at 212°F (100°C) Tensile strength, loss, max, % Elongation, loss, max, % Hardness, Type A durometer, points change	20 max 20 max 0 to + 10	D 573
Oil swell, ASTM Oil No. 3, 70 h at 212°F (100°C) Weight change, max, %	45 max	D 471
Ozone resistance 20% strain, 300 pphm in air, 70 h at 104°F (40°C)	no cracks	D 1149 (modified) ^B
Low-temperature stiffening, 7 days at 14°F (-10°C) Hardness, Type A durometer, points change	0 to + 15	D 2240
Low-temperature recovery, ^C 72 h at 14°F (-10°C) 50% deflection, min, %	88	9.2 ^D
Low-temperature recovery, ^C 22 h at -20°F (-29°C), 50% deflection, min, %	83	9.2 ^D
High-temperature recovery, ^C 70 h at 212°F (100°C) 50% deflection, min, %	85	9.2 ^D
Compression-deflection, at 80% of nominal width, min, lbf/in. (N/m)	3.5 (613)	9.3 ^D

A The term “modified” in the table relates to the specimen preparation. The use of joint seal as the specimen source requires that more plies than specified in either of the modified test procedures be used. Such specimen modification shall be agreed upon by the purchaser and seller prior to testing. The hardness test shall be made with the durometer in a durometer stand as recommended in Method D 2240.

B Test in accordance with Procedure A of D 518.

C Cracking, splitting, or sticking of a specimen during a recovery test shall mean that the specimen has failed the test.

D The reference sections are those of this specification.



Delastic® Pavement Seals were successfully used on Route 407 near Toronto, Ontario.